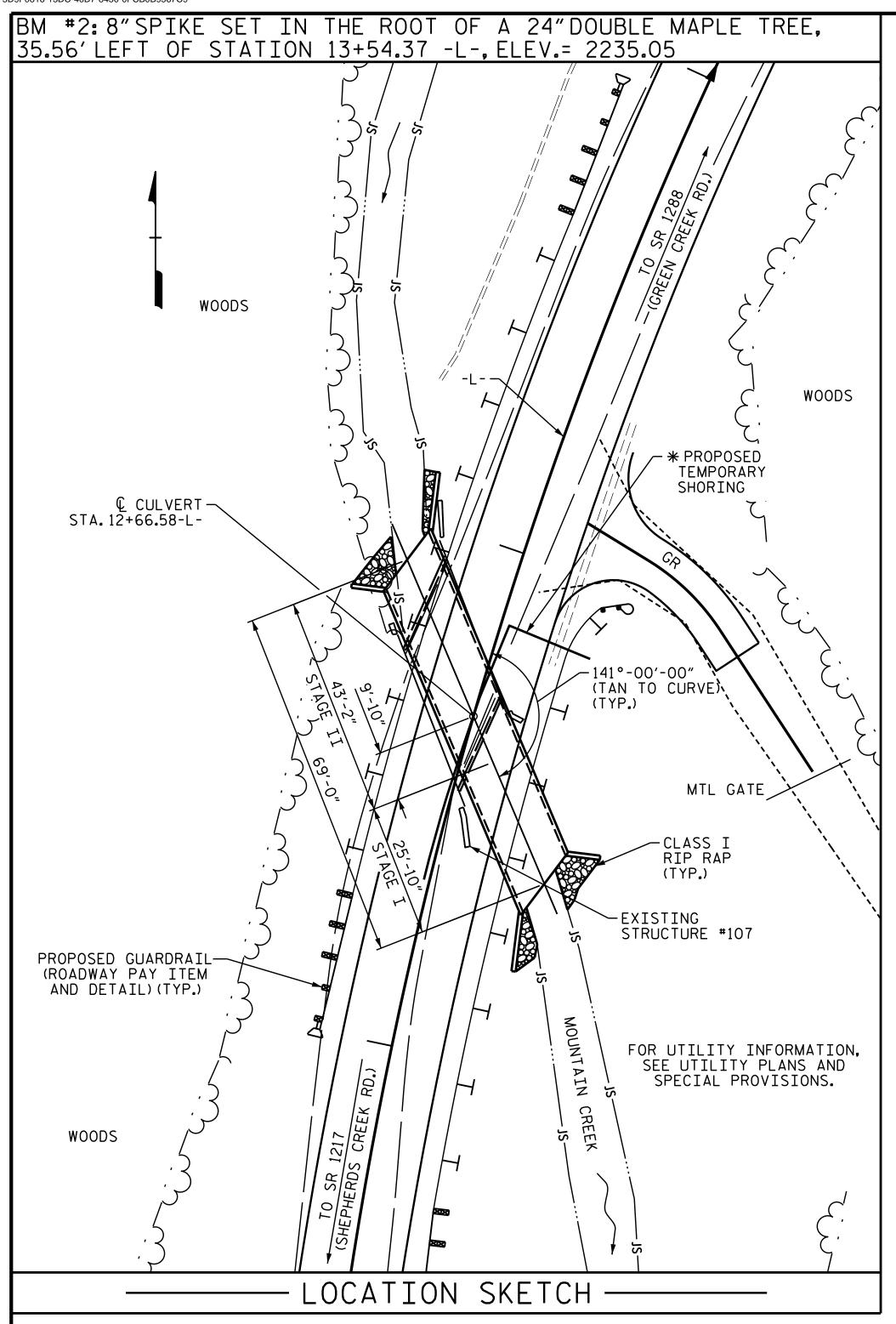
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NOTES

ASSUMED LIVE LOAD......HL-93 OR ALTERNATE LOADING.

MAXIMUM DESIGN FILL......1.4'

MINIMUM DESIGN FILL......1.12'

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTE SHEET.

FOR CULVERT DIVERSION DETAILS, SEE EROSION CONTROL PLANS.

3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS AND FLOOR SLAB FOR STAGE I, INCLUDING 4"OF ALL VERTICAL WALLS OF STAGE I.

2. THE REMAINING PORTIONS OF THE WALLS OF STAGE I AND WINGS FULL HEIGHT.

3. ROOF SLAB AND HEADWALLS OF STAGE I.

4. WING FOOTING AND FLOOR SLAB FOR STAGE II, INCLUDING 4"OF ALL VERTICAL WALLS OF STAGE II.

5. THE REMAINING PORTIONS OF THE WALLS OF STAGE II AND WINGS FULL HEIGHT.

6. ROOF SLAB AND HEADWALLS OF STAGE II.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF A SINGLE SPAN: 20'-8" WITH AN ASPHALT WEARING SURFACE ON A TIMBER DECK AND TIMBER JOISTS HAVING A CLEAR ROADWAY WIDTH OF 20'-5" SUPPORTED ON A SUBSTRUCTURE OF TIMBER CAPS, TIMBER POSTS, AND MASS CONCRETE SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED CULVERT, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF THE EXTERIOR WALL ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

* FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE ROADWAY PLANS.

STEEL SHEET PILING REQUIRED FOR SHORING SHALL BE HOT-ROLLED.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

THE EXISTING BRIDGE SHALL BE PARTIALLY REMOVED AS SHOWN IN THE TRAFFIC CONTROL PLANS. THE EXISTING RAIL SHALL BE REMOVED AND REATTACHED TO THE REMAINING PORTION OF THE EXISTING BRIDGE. SEE SPECIAL PROVISION FOR "REMOVAL OF EXISTING STRUCTURE".

EXCAVATE A MINIMUM OF 1 FOOT BELOW CULVERT BEARING ELEVATION AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL PER SECTION 414 OF THE STANDARD SPECIFICATIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF THE CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

CLASS A CONCRETE STAGE I STAGE II G1.4 CU. YDS. STAGE III G1.4 CU. YDS. TOTAL REINFORCING STEEL STAGE I STAGE II G.956 LBS. STAGE II 10,658 LBS. TOTAL CULVERT EXCAVATION FOUNDATION COND. MAT'L STAGE I STAGE II 41 TONS TOTAL CENTRAL STAGE II 41 TONS TOTAL G66 TONS REMOVAL OF EXISTING STRUCTURE CUMP SUM GEOTEXTILE FOR DRAINAGE STAGE II STAGE II STAGE II G6 TONS CLASS I RIP RAP STAGE II	TOTAL STRUCTURE	QUANTITIES
STAGE I STAGE II STAGE II 10,658 LBS. TOTAL 17,614 LBS. CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L STAGE I STAGE II 41 TONS TOTAL 66 TONS REMOVAL OF EXISTING STRUCTURE LUMP SUM GEOTEXTILE FOR DRAINAGE STAGE II 93 SO.YDS. STAGE II 93 SO.YDS. CLASS I RIP RAP STAGE II 5 TONS TOTAL 11 TONS	STAGE II	61.4 CU. YDS.
FOUNDATION COND. MAT'L STAGE I STAGE II 41 TONS TOTAL 66 TONS REMOVAL OF EXISTING STRUCTURE LUMP SUM GEOTEXTILE FOR DRAINAGE STAGE I STAGE II 93 SO.YDS. STAGE II 93 SO.YDS. TOTAL 176 SO.YDS. CLASS I RIP RAP STAGE II 5 TONS TOTAL 11 TONS	STAGE II	10,658 LBS.
STAGE I STAGE II STAGE II STAGE II STAGE II STAGE II STOTAL REMOVAL OF EXISTING STRUCTURE LUMP SUM GEOTEXTILE FOR DRAINAGE STAGE I STAGE II STAG	CULVERT EXCAVATION	LUMP SUM
GEOTEXTILE FOR DRAINAGE STAGE I 83 SQ.YDS. STAGE II 93 SQ.YDS. TOTAL 176 SQ.YDS. CLASS I RIP RAP STAGE I 6 TONS STAGE II 5 TONS TOTAL 11 TONS	STAGE I STAGE II	41 TONS
STAGE II STA	REMOVAL OF EXISTING STRUCTURE	LUMP SUM
STAGE I 6 TONS STAGE II 5 TONS TOTAL 11 TONS	STAGE I STAGE II	93 SQ.YDS.
TEMPORARY EDGE BEAM LUMP SUM	STAGE II	5 TONS
	TEMPORARY EDGE BEAM	LUMP SUM

HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD = <250 CFS = <2 YEARS DESIGN HIGH WATER ELEVATION = 2234.0 DRAINAGE AREA = 2.8 SQ.MI. BASE DISCHARGE (Q100) = 1200 CFS = 2236.28 BASE HIGH WATER ELEVATION

OVERTOPPING DATA

OVERTOPPING DISCHARGE = 230 CFS FREQUENCY OF OVERTOPPING FLOOD = <2 YEARS OVERTOPPING FLOOD ELEVATION = 2233.9 @ US FACE OF CULVERT

HORIZONTAL CURVE DATA

PI STA 12+72.79 △ = 28° 42′56.2″(RT) D = 6° 30′ 00.0″ L = 441.78'T = 225.63' R = 881.47'

GRADE DATA

GRADE POINT ELEV. @ STA. 12+66.58-L-INVERT ELEV. @ STA. 12+66.58-L-ROADWAY SIDE SLOPES

= 2233.33 = 2227.08 = 2:1



1/6/2016

WSP

ransportation & Infrastructure

15401 Weston Parkway Suite 100 Cary, NC 27513 - 919.678.0035

www.wspgroup.com LICENSE NO. F-0891

DESIGN ENGINEER OF RECORD:

Nicholas Pierce

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

GRAHAM COUNTY 12+66.58-L-SHEET 1 OF 6 REPLACES BRIDGE #10

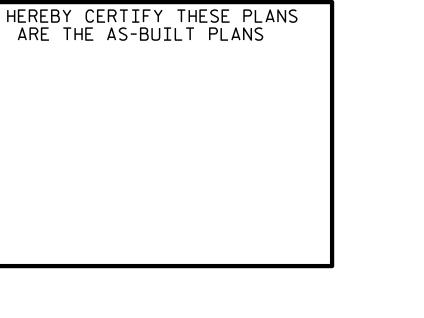
PROJECT NO. 17BP.14.R.90

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RALEIGH

SINGLE 12 FT. X 4.0 FT CONCRETE BOX CULVERT 141° SKEW

SHEET NO REVISIONS C-1 NO. DATE: DATE: BY: BY: TOTAL SHEETS

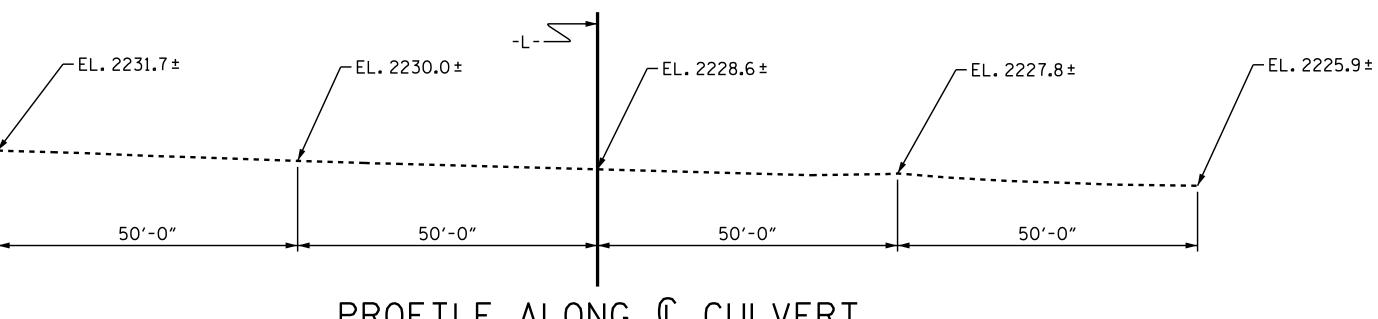


N. PIERCE

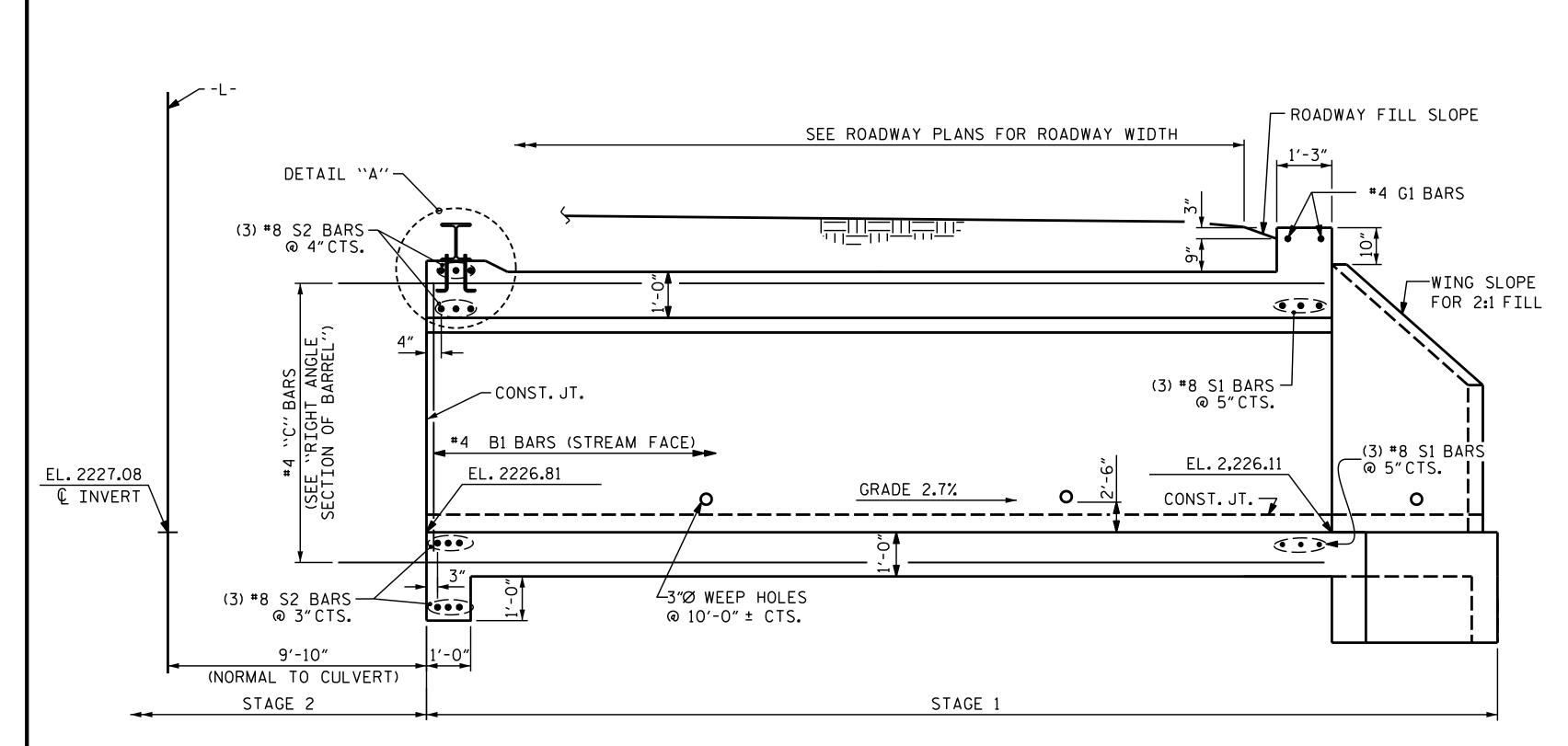
CHECKED BY : M. HOBBS

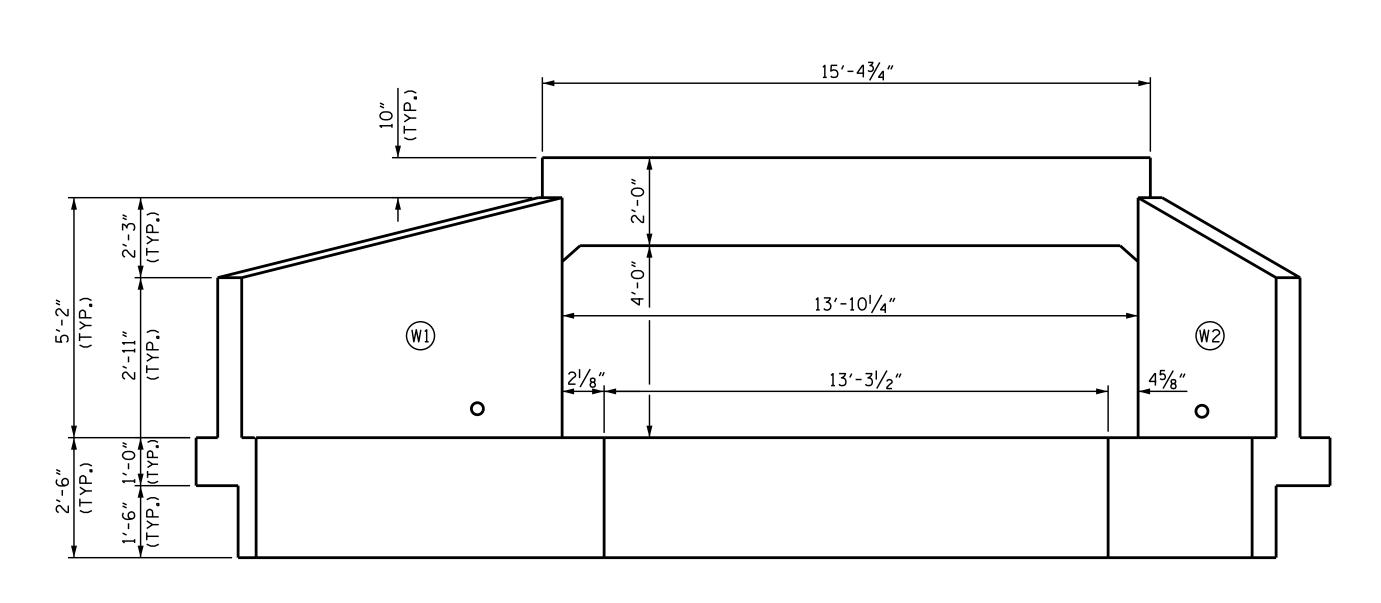
_ DATE : <u>07/2014</u>

DATE : 08/2014



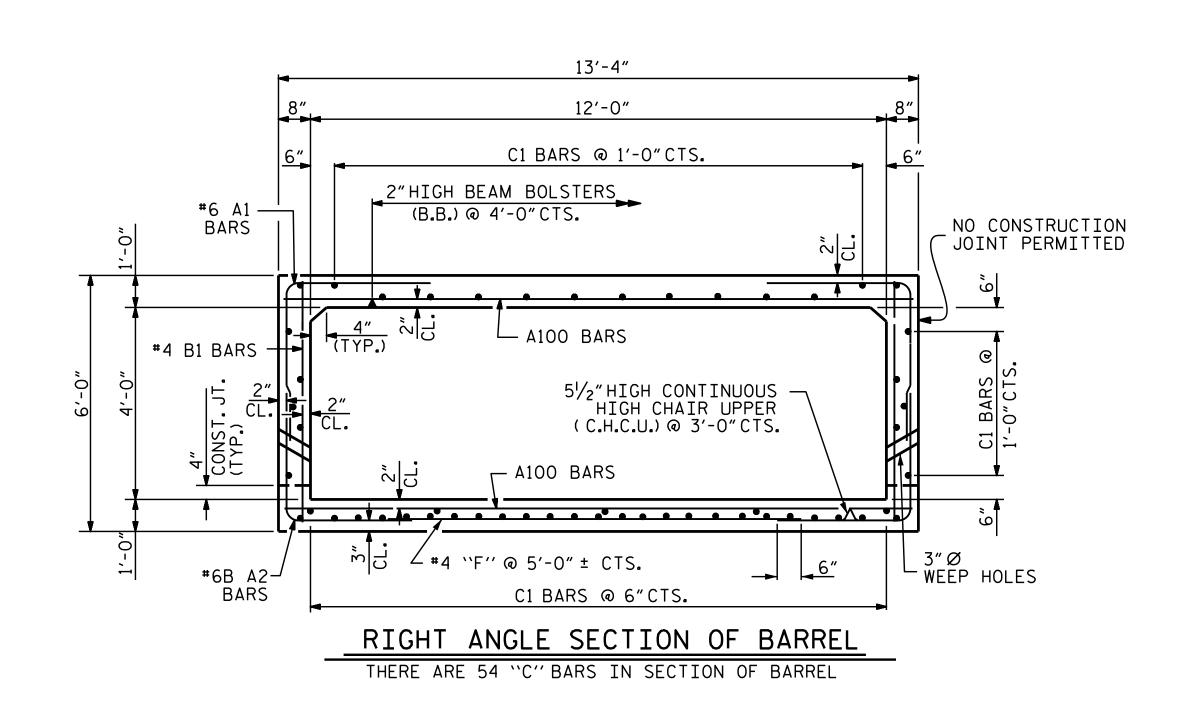
PROFILE ALONG & CULVERT

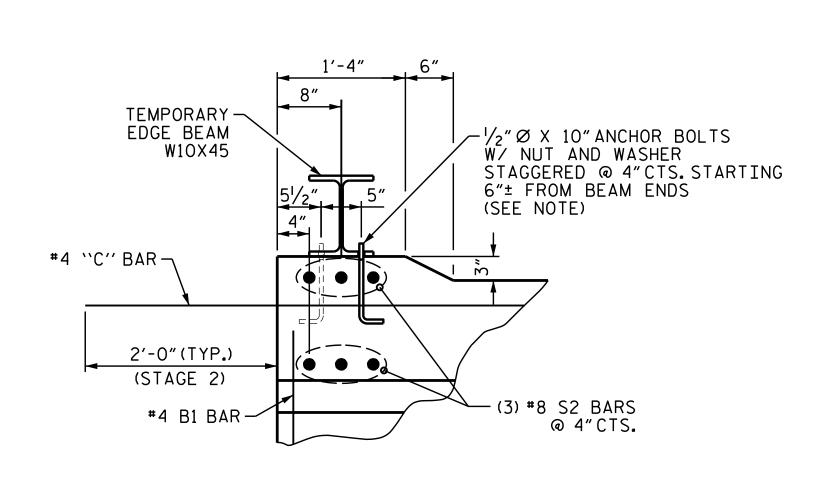




CULVERT SECTION NORMAL TO ROADWAY (STAGE 1)

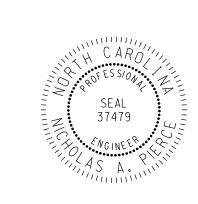
END ELEVATION NORMAL TO HEADWALL SKEW





NOTE: AFTER STAGE 2 IS COMPLETE, THE BEAM SHALL BE REMOVED AND ANCHOR BOLTS CUT SMOOTH AND EPOXY APPLIED TO EXPOSED BOLTS TO PREVENT CORROSION.

DETAIL "A"



DESIGN ENGINEER OF RECORD: Nicholas Pierce

1/6/2016

WSP Transportation & Infrastructure 15401 Weston Parkway Suite 100 Cary, NC 27513 - 919.678.0035

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED PROJECT NO. <u>17BP.14.R.90</u> GRAHAM STATION: 12+66.58 -L-

SHEET 2 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

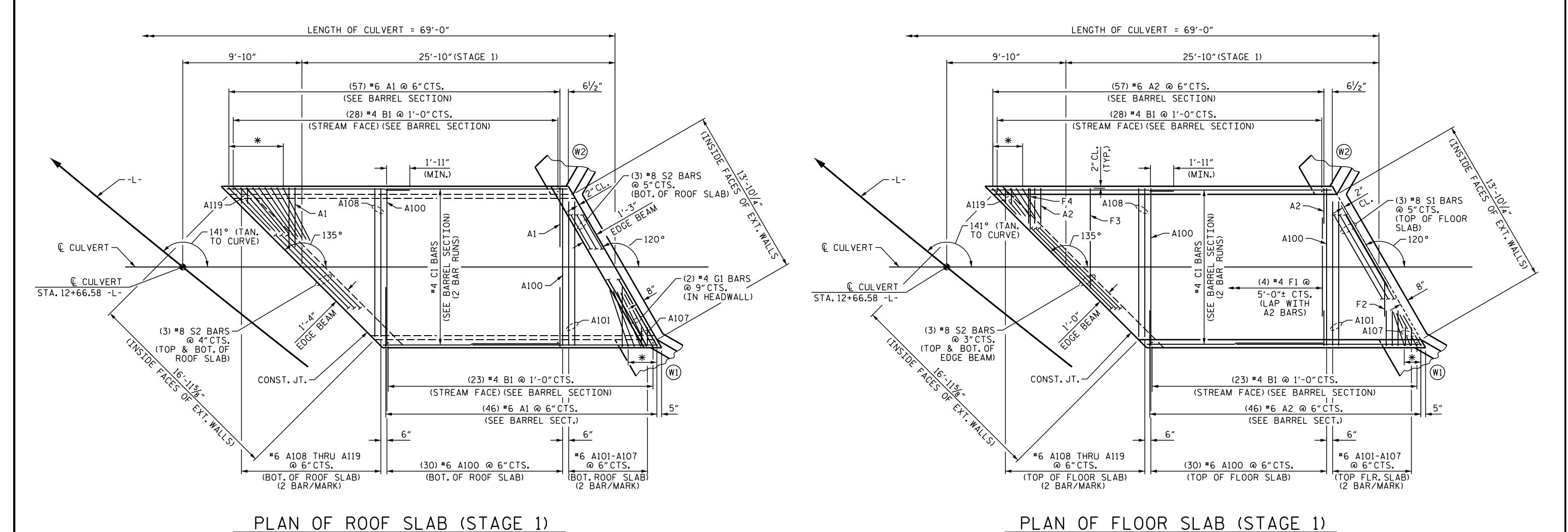
HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

SINGLE 12FT. X 4FT. CONCRETE BOX CULVERT STAGE 1

SHEET NO. **REVISIONS** NO. BY: C-2 DATE: BY: DATE: TOTAL SHEETS

M. HOBBS DATE: 08/2014
N. PIERCE DATE: 08/2014 CHECKED BY : _

www.wspgroup.com LICENSE NO. F-0891

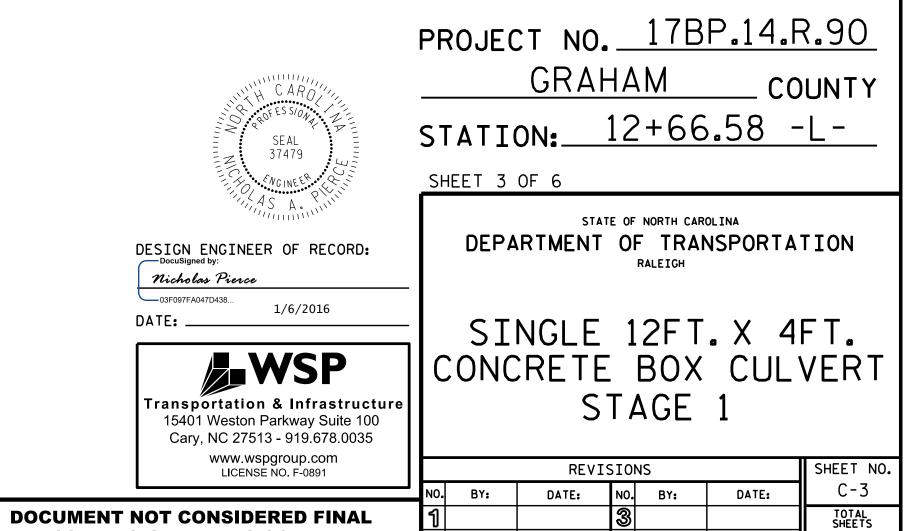


(FOR S1 BARS IN FLOOR SLAB & WING FOOTINGS, SEE WING SHEET)

(ALL "F" BARS ARE SPACED 5'-O"± CTS. AND ADJUSTED AS NEEDED TO TIE TO "A2" BARS)

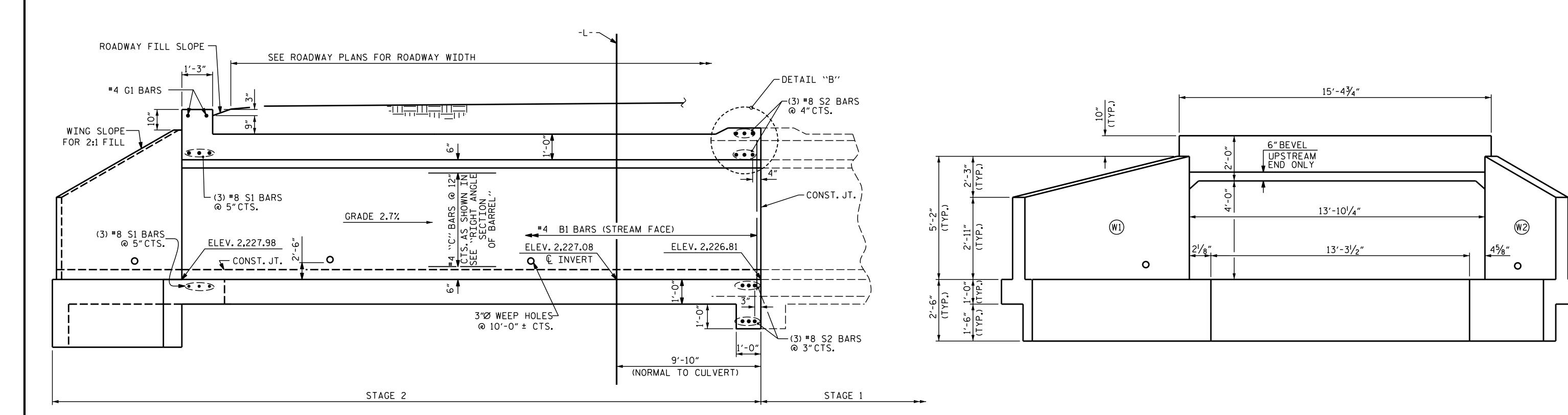
(** DENOTES SPLAYED "A2" BARS @ 6"CTS. WITH 2"MIN. CLEAR AT BAR ENDS)

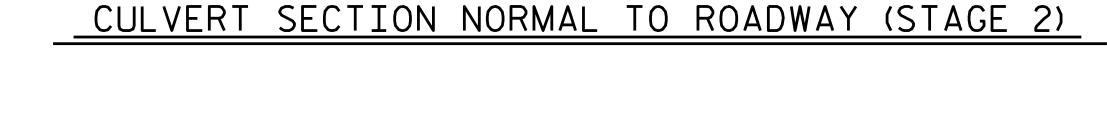
UNLESS ALL SIGNATURES COMPLETED



DRAWN BY: M. HOBBS DATE: 08/2014
CHECKED BY: N. PIERCE DATE: 08/2014

(* DENOTES SPLAYED "A1" BARS @ 6"CTS. WITH 2"MIN. CLEAR AT BAR ENDS)

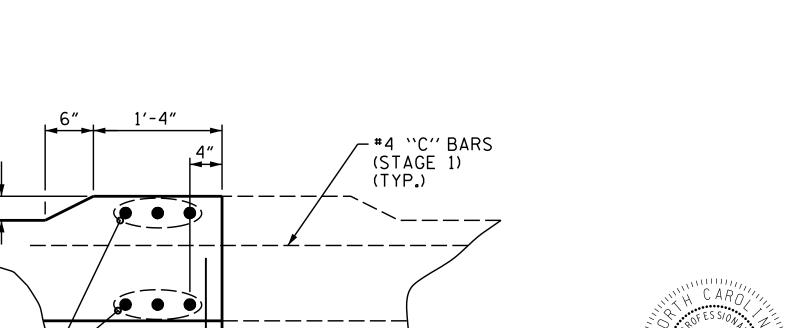




13'-4"

12'-0"

C1 BARS @ 1'-0"CTS.



DESIGN ENGINEER OF RECORD:

1/6/2016

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Nicholas Pierce

DATE: ___

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

END ELEVATION NORMAL TO HEADWALL SKEW

PROJECT NO. <u>17BP.14.R.90</u> GRAHAM COUNTY STATION: 12+66.58 -L-

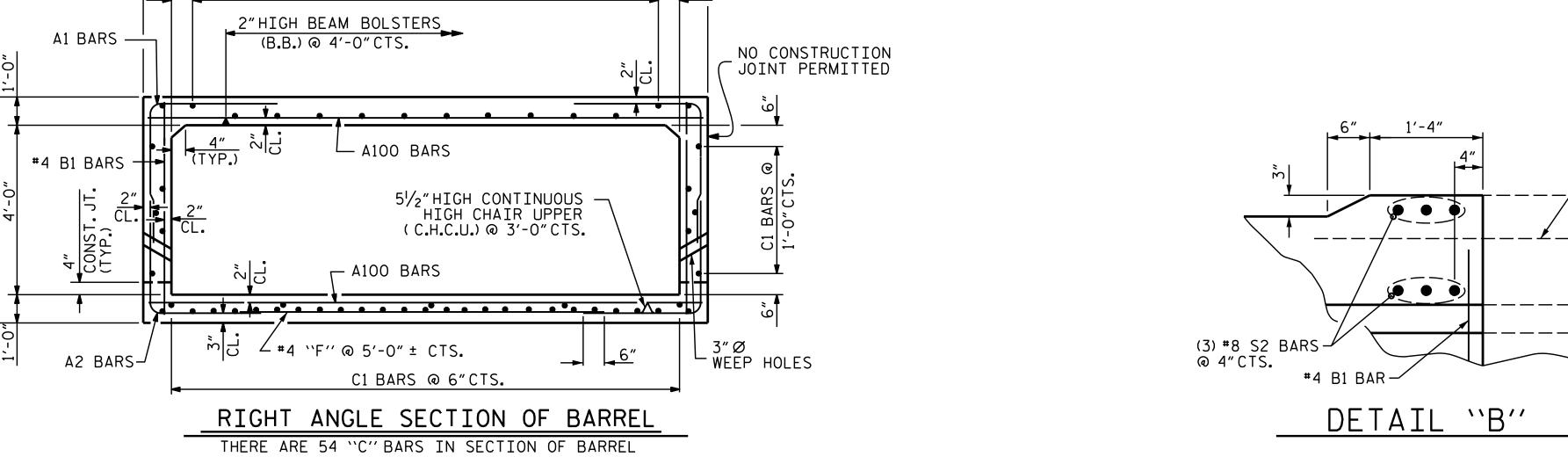
SHEET 4 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

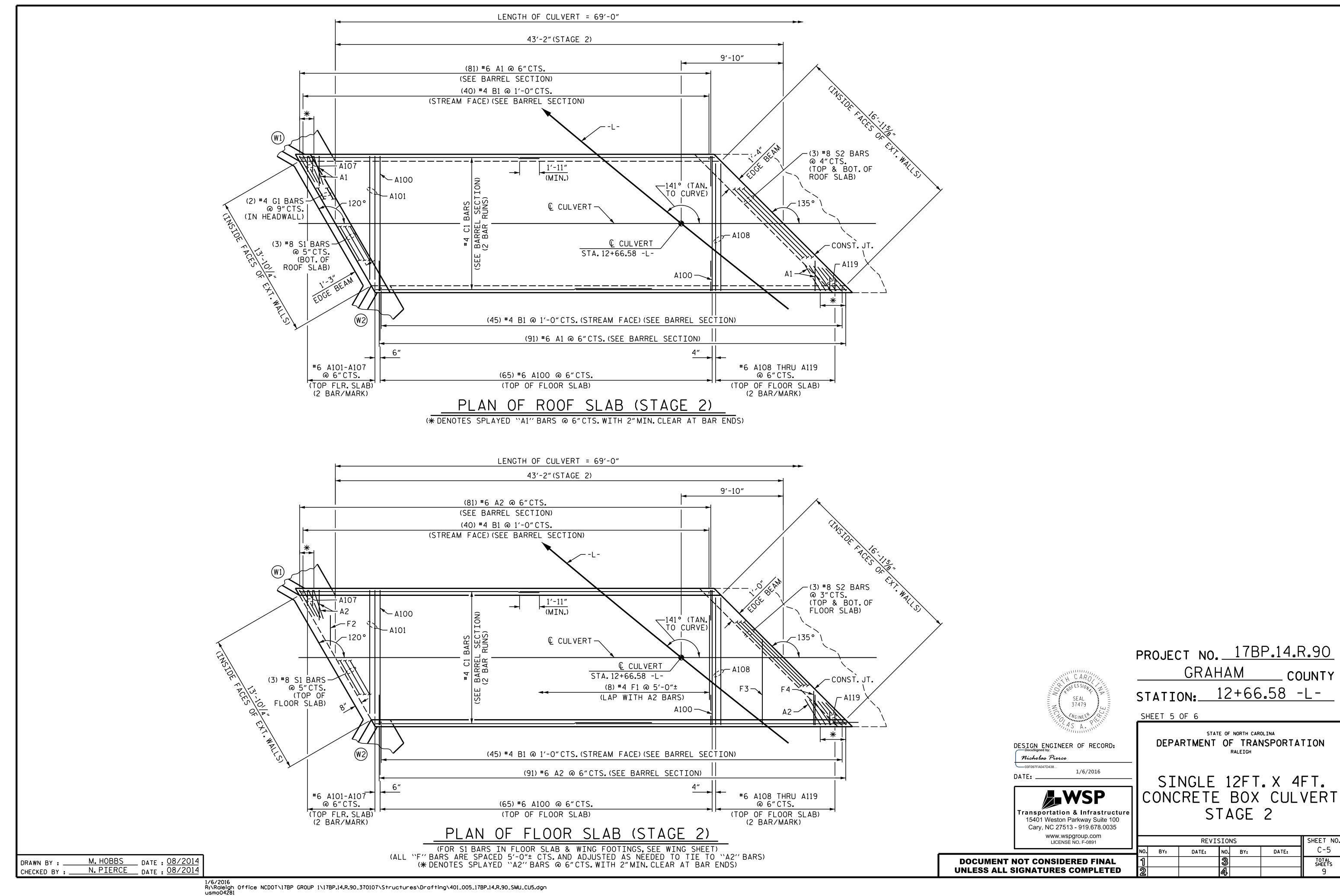
I HEREBY CERTIFY THESE PLANS
ARE THE AS-BUILT PLANS

SINGLE 12FT.X 4FT. CONCRETE BOX CULVERT STAGE 2 Transportation & Infrastructure 15401 Weston Parkway Suite 100

> SHEET NO. REVISIONS NO. BY: C-4 BY: DATE: TOTAL SHEETS



M. HOBBS DATE: 08/2014
N. PIERCE DATE: 08/2014 ASSEMBLED BY :



NOTES

NATIVE MATERIAL EXCAVATED FROM THE EXISTING STREAM BED OR FLOOD PLAIN AT THE PROJECT SITE DURING CULVERT CONSTRUCTION SHALL BE STOCKPILED AND LATER PLACED IN THE PROPOSED CULVERT BETWEEN SILLS TO PROVIDE A CONTINUOUS LOW FLOW CHANNEL. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

THE STOCKPILED NATIVE MATERIAL SHALL BE PLACED AS SHOWN IN THE "PLAN OF FLOOR SILL LAYOUT" SKETCH TO PROVIDE A 1'-6" DEPTH LOW FLOW CHANNEL BETWEEN LOW FLOW SILLS, AND SHALL BE PLACED TO THE DEPTH OF 2'-O"BETWEEN HIGH FLOW SILL.

SUPPLEMENTAL STONE OF SIMILAR CHARACTERISTICS OF THE NATIVE MATERIAL MAYBE USED AS NECESSARY WITH APPROVAL BY ENGINEER.

THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION.

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

TOP OF LOW FLOW SILLS SHOULD MATCH STREAM BED ELEVATION IN LOW FLOW CHANNEL OF STREAM. (THALWEG)

DO NOT SET ELEVATION OF HIGH SILL ABOVE BANK FULL.

NUMBER OF SILLS DETERMINED BY THE ENGINEER.

CLASS A CONCRETE BARREL @ 1.1926 CY/FT 30.8	
BARREL @ 11926 CY/ET 30.8	
DAINILL W	CY
WINGS, ETC. 7.7	CY
SILLS	CY
TOTAL40.0	CY
REINFORCING STEEL	
BARREL & SILLS 6,634	LBS.
WINGS, ETC. 322	LBS.
TOTAL6,956	LBS.
* CULVERT EXCAVATION LUMP S	SUM
FOUNDATION COND. MAT'L 25 TO)NS

STAGE 2 QUANTITIES		
CLASS A CONCRETE BARREL @1.1926 CY/FT WINGS, ETC STLIS	51.5 7.7 2.2	CY
SILLSTOTAL	61.4	_
REINFORCING STEEL		
BARREL & SILLS	10,336	_LBS.
WINGS, ETC	322	_LBS.
TOTAL	10,658	_ LBS.
* CULVERT EXCAVATION	LUMP	SUM
FOUNDATION COND. MAT'L	41 T	ONS
A CHILDERT EVOLUATION DAY TIEM DAID ONCE FOR THE		

*CULVERT EXCAVATION PAY ITEM PAID ONCE FOR THE PROJECT.



A119 | 4 |

D2

G1

F1

F2

F3

F4

4

4

2

4

REINFORCING STEEL

#6 | STR | 1'-3"

#6 | STR | 2'-7"

#6 | STR | 2'-1"

#4 | STR | 15'-0"

#4 | STR | 8'-9"

#4 | STR | 5'-0"

#8 | STR | 15'-0"

SEAL 37479

4/20/2016

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8′-3″

3′-3"

#4 | STR |

#4 | STR |

S2 | 12 | #8 | STR | 18'-4"

B1 | 51 | #4 | STR | 5'-8"

C1 | 108 | #4 | STR | 16'-1"

12'-0" SPAN 8'-0" HIGH SILL -·2 LAYERS OF 30 LB. ∕-LOW SILL ROOFING FELT TO PREVENT BONDING (TYP.) -D1 #6 DOWELS (TYP.EA.END) (4) #6 "D" BARS @ →D2 #6 DOWELS 3'-4" CTS. ELEVATION LOOKING DOWNSTREAM 1'-0" — * *6 DOWEL (TYP.) AT A MAX. SPACING OF 4'-0"

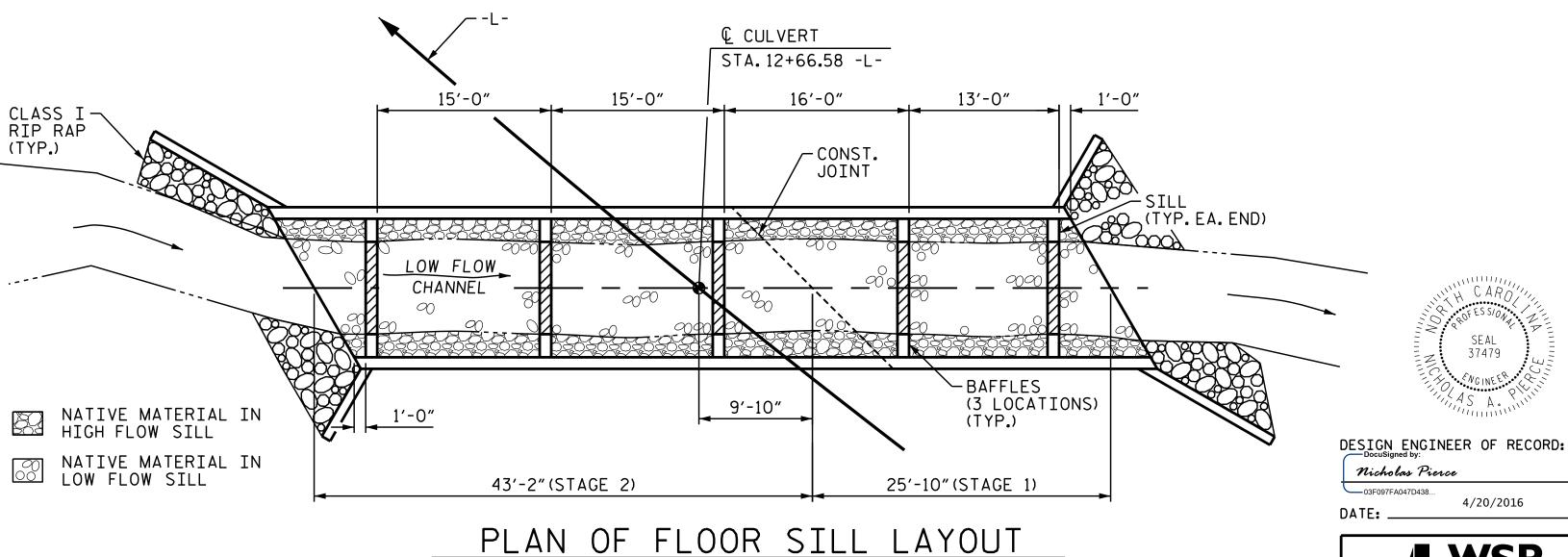
2 LAYERS OF 30 LB.

PREVENT BOND (TYP.)

ROOFING FELT TO

SECTION THROUGH SILL * DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOAT FINISHED

CULVERT SILL DETAILS



LOW FLOW SILLS DENOTED BY CROSSHATCHED AREA.

PROJECT NO. 17BP.14.R.90 GRAHAM __ COUNTY

8'-10" | 2282

1765

72

61

51

40

30

20

9

74

68

62

56

50

44

38

32

26

20

14

8

322

23

19

20

47

6

240

587

6'-10"

13′-0″

11'-11"

10'-2"

8′-5″

6′-8″

4'-11"

3′-3″

1′-6″

12′-3″

11'-3"

10′-3″

9′-3″

8′-3″

7′-3"

6′-3"

5′-3"

4′-3″

3′-3"

2′-3″

1'-3"

5′-8″

2'-1"

15′-0″

5′-0"

8′-3″

3′-3"

15'-0"

18′-4″

LBS. 10.336

#6 STR

#6 | STR

#4 | STR |

#4 | STR

#4 | STR

#4 | STR

#8 | STR |

#6 | STR | 2'-7"

#4 | STR | 8'-9"

| A119 | 4 |

D1

D2

G1

F2

F3

F4

S1

6.634 REINFORCING STEEL

23 F1

▮ B1 | 85 | #4 | STR |

6

6

2

8

6

| S2 | 12 | #8 | STR |

8

193

16

13

20

3

240

587

STATION: 12+66.58 -L-

SHEET 6 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

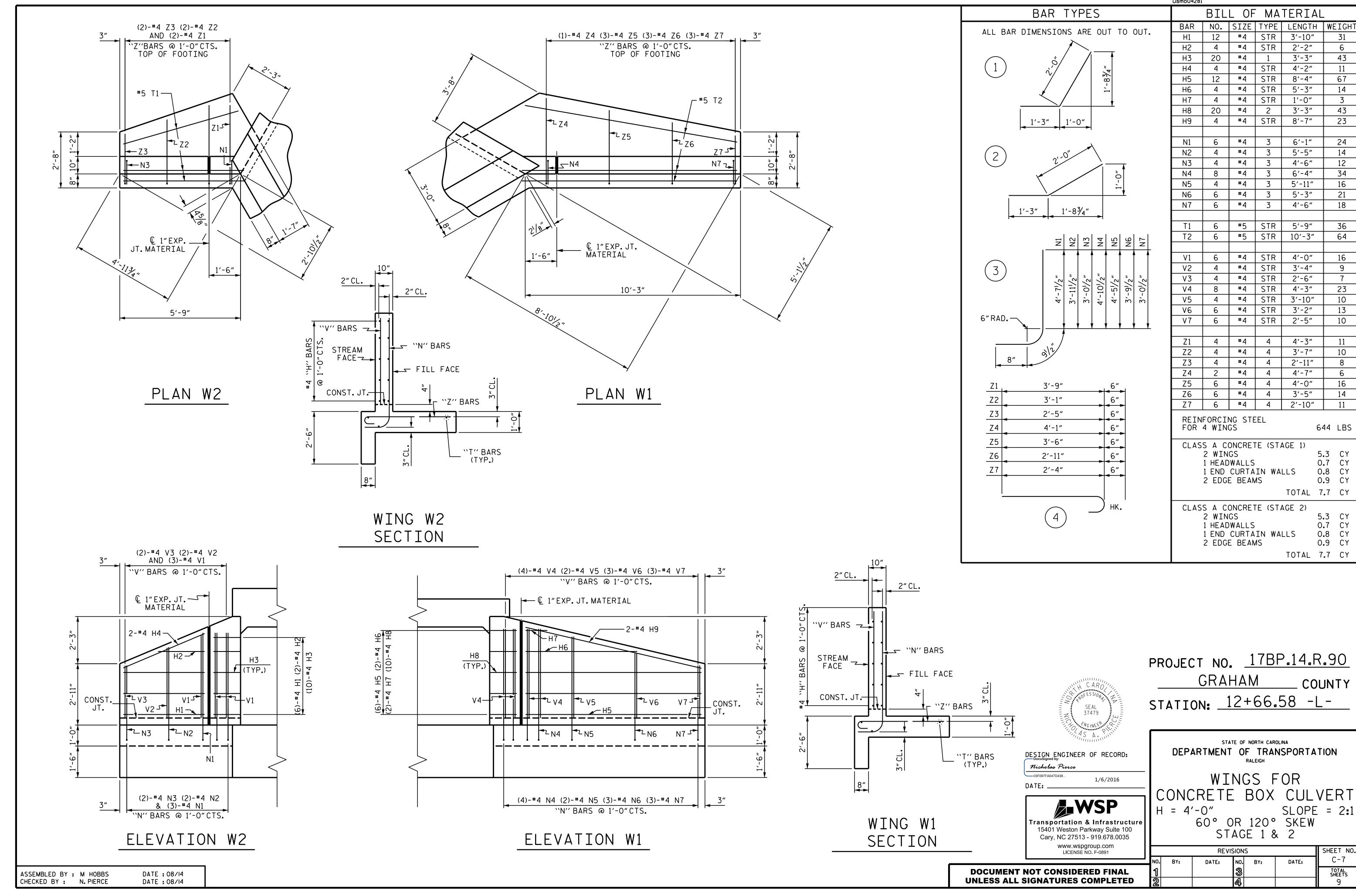
SINGLE 12FT. X 4FT. CONCRETE BOX CULVERT STAGES 1 & 2 BILL OF MATERIAL

SHEET NO. **REVISIONS** C-6 NO. DATE: DATE: BY: BY: TOTAL SHEETS

M. HOBBS DATE: 08/2014
N. PIERCE DATE: 08/2014

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1/6/2016
R:\Raleigh Office NCDOT\17BP GROUP 1\17BP.14.R.90_370107\Str
usmo04281



€ CULVERT -

DATE: 08/2014 DATE: 08/2014

RWW/JTE

KMM/GM

ASSEMBLED BY : M. HOBBS CHECKED BY : N. PIERCE

DRAWN BY: FCJ 6/88 REV. 7/10/01 REV. 5/7/03 REV. 5/1/06R

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".
- B. $4 1'' \varnothing \times 2^{1}/_{4}''$ BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 21/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A 7_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.

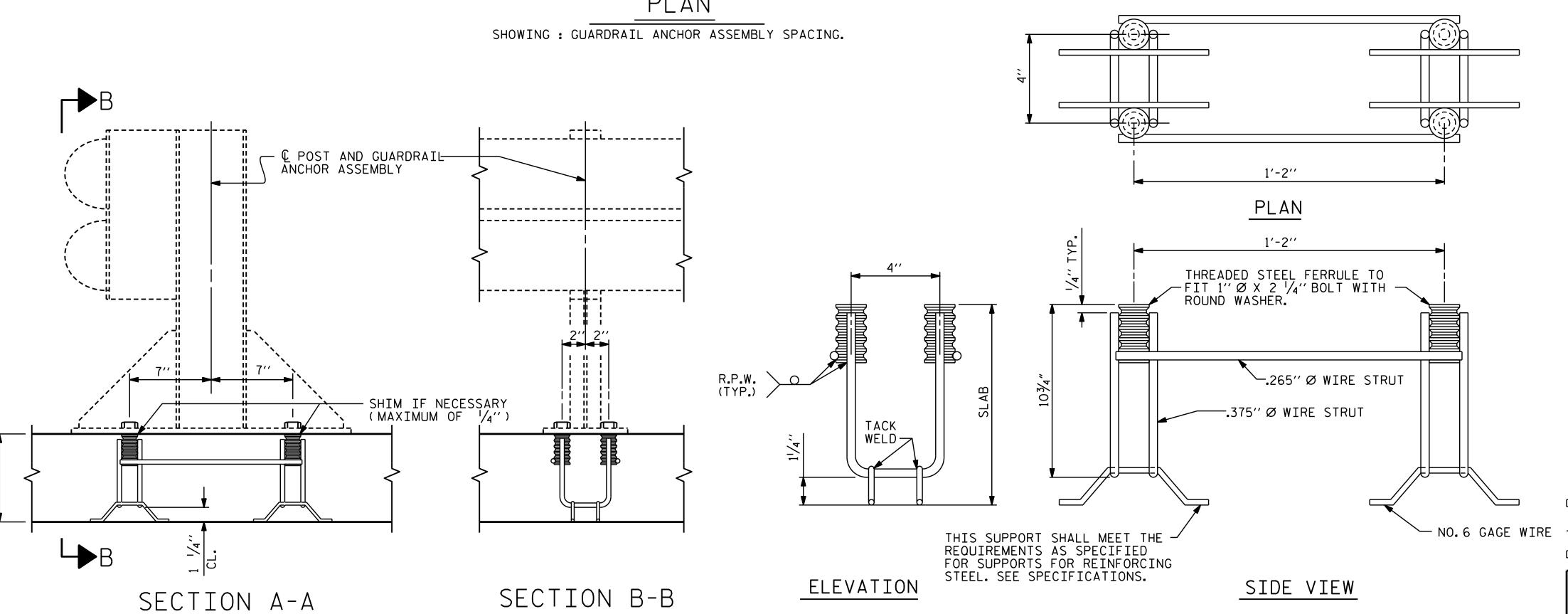
AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.

PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

FOR CULVERTS 24FT OR LESS, SEE ROADWAY STD 862.01 SHEET 10 OF 12 AS OPTIONAL GUARDRAIL PLACEMENT.



69'-0"

🖟 POST AND GUARDRAIL-

----+-----

ANCHOR ASSEMBLY (TYP.)

-141°-00′-00″ (TAN. TO CURVE)

(TYP.)

> 33′−4″

·----

© CULVERT-STA.12+66.58-L-

.20°-00′-`00"

35'-8"

DIRECTION OF FLOW

GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

-120°-00′-00″

PROJECT NO. 17BP.14.R.90 GRAHAM STATION: 12+66.58 -L-

DESIGN ENGINEER OF RECORD: Nicholas Pierce

1/6/2016

WSP Transportation & Infrastructure 15401 Weston Parkway Suite 100 Cary, NC 27513 - 919.678.0035 www.wspgroup.com LICENSE NO. F-0891

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

STANDARD

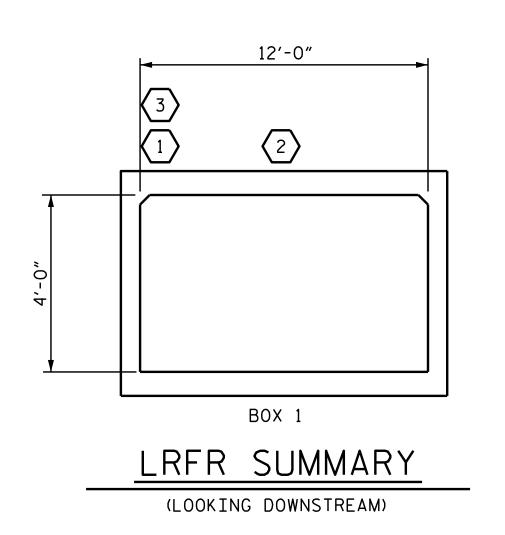
ANCHORAGE DETAILS FOR GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

SHEET NO **REVISIONS** C-8 DATE: DATE: BY: BY: TOTAL SHEETS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

							STRENGTH I LIMIT STATE									
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.28		1.75	1.36	1	TOP SLAB	6.67	1.28	1	TOP SLAB	0.67	
DESIGN LOAD		HL-93 (OPERATING)	N/A		1.65		1.35	1.77	1	TOP SLAB	6.67	1.65	1	TOP SLAB	0.67	
RATING		HS-20 (INVENTORY)	36.000	2	1.40	50.40	1.75	1.40	1	TOP SLAB	6.67	1.75	1	TOP SLAB	0.67	
		HS-20 (OPERATING)	36.000		1.82	65.52	1.35	1.82	1	TOP SLAB	6.67	2.26	1	TOP SLAB	0.67	
	: VEHICLE (SV)	SNSH	13.500		2.55	34.43	1.40	2.55	1	TOP SLAB	6.67	3 . 55	1	TOP SLAB	0.67	
		SNGARBS2	20.000		2.39	47.80	1.40	2.39	1	TOP SLAB	6.67	3.27	1	TOP SLAB	0.67	
		SNAGRIS2	22.000		2.55	56.10	1.40	2.55	1	TOP SLAB	6.67	3 . 55	1	TOP SLAB	0.67	
	VEH.	SNCOTTS3	27.250	3	1.59	43.33	1.40	1.70	1	TOP SLAB	6.67	1.59	1	TOP SLAB	0.67	
	LE (S	SNAGGRS4	34.925		1.87	65.31	1.40	1.87	1	TOP SLAB	6.67	2.00	1	TOP SLAB	0.67	
	SINGLE (§	SNS5A	35.550		1.85	63.92	1.40	1.85	1	TOP SLAB	6.67	1.88	1	TOP SLAB	0.67	
	"	SNS6A	39.950		1.86	74.31	1.40	1.86	1	TOP SLAB	6.67	1.92	1	TOP SLAB	0.67	
LEGAL		SNS7B	42.000		1.86	78.12	1.40	1.86	1	TOP SLAB	6.67	1.86	1	TOP SLAB	0.67	
LOAD RATING	ER	TNAGRIT3	33.000		2.55	84.15	1.40	2.55	1	TOP SLAB	6.67	3.02	1	TOP SLAB	0.67	
	RAII	TNT4A	33.075		2.03	67.14	1.40	2.03	1	TOP SLAB	6.67	2.03	1	TOP SLAB	0.67	
	SEMI-TRAILER T)	TNT6A	41.600		1.86	77.38	1.40	1.86	1	TOP SLAB	6.67	1.88	1	TOP SLAB	0.67	
	SEN ST)	TNT7A	42.000		1.94	81.48	1.40	1.94	1	TOP SLAB	6.67	1.95	1	TOP SLAB	0.67	
	TRACTOR S	TNT7B	42.000		1.86	78.12	1.40	1.86	1	TOP SLAB	6.67	1.92	1	TOP SLAB	0.67	
		TNAGRIT4	43.000		2.03	87.29	1.40	2.03	1	TOP SLAB	6.67	2.03	1	TOP SLAB	0.67	
	TRUCK	TNAGT5A	45.000		2.02	90.90	1.40	2.03	1	TOP SLAB	6.67	2.02	1	TOP SLAB	0.67	
	TRL	TNAGT5B	45.000		2.02	90.90	1.40	2.03	1	TOP SLAB	6.67	2.02	1	TOP SLAB	0.67	



DATE : 8/14 DATE : 8/14 C. HOWARD N. PIERCE ASSEMBLED BY : CHECKED BY : DRAWN BY: WMC 7/II
CHECKED BY: GM 7/II

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1.50	0.65		
EV	1.30	0.90		
ЕН	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

1/6/2016

PROJECT NO. 17BP.14.R.90

GRAHAM COUNTY

STATION: 12+66.58 -L-

DESIGN ENGINEER OF RECORD: Nicholas Pierce

WSP

Transportation & Infrastructure 15401 Weston Parkway Suite 100 Cary, NC 27513 - 919.678.0035 www.wspgroup.com LICENSE NO. F-0891

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)

STATE OF NORTH CAROLINA

REVISIONS SHEET NO. C-9 NO. BY: DATE: DATE: BY: TOTAL SHEETS 9

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER - - - -EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS. SETTLEMENT OF FALSEWORK. AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $rac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH